## MEMORANDUM

To: The City of Panama City Beach
From: Kimley-Horn and Associates
Date: November 15, 2022

## Subject: Hombre Apartments Traffic Analysis

## Purpose and Methodology

The purpose of this memorandum is to summarize the turn lane analysis prepared for the proposed multifamily housing development located southwest of the intersection of US 98 and N Glades Trail in Panama City Beach, Florida. The proposed development has two access points; the US 98 access is a right-in/right-out driveway, and the N Glades Trail access is a full access driveway. There is an additional access driveway on Coyote Pass which is not included in the analysis because of its restriction for emergency vehicles only. This memorandum provides the methodology and analysis for evaluating the need for an eastbound right-turn lane on US 98, as well as a northbound left-turn lane and a southbound right-turn lane on N Glades Trail. A map of the project location is shown below in Figure 1, and a site plan for the proposed development is available in Attachment A.

Figure 1: Project Location


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## Trip Generation

The subject development is proposed to consist of 267 multifamily units. The Institute of Transportation Engineers (ITE) Trip Generation Manual, $11^{\text {th }}$ Edition was utilized to calculate the trip generation potential of the proposed development. ITE trip generation rates for Land Use Code 220 (Multifamily Housing [Low-Rise]) were applied to calculate the daily and PM peak hour trips anticipated from the proposed development. The development is expected to generate approximately 1,787 daily net new trips and 135 PM peak hour net new trips ( 85 entering, 50 exiting). Table 1 summarizes the trip generation calculations.

Table 1: Trip Generation Calculations

| ITE | Land Use | Density |  | Daily <br> Trips | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LUC |  |  |  | Total | In | Out |
| 220 | Multifamily Housing (Low-Rise) | 267 | Units |  | 1,787 | 135 | 85 | 50 |
| GROSS TRIPS 1,787 135 85 50 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |

## Traffic Development

Existing traffic volumes were obtained from the Florida Department of Transportation (FDOT) Florida Traffic Online (2021) and from a previously submitted FDOT intersection analysis report completed at the site. The peak directional volumes for US 98 were retrieved from count station 460203, just east of the site. The seasonal factor corresponding to the week of data collection for US 98 was 0.99 . Since the seasonal factor was less than 1.0, to provide for a conservative analysis, it was not applied. Historical traffic data from surrounding count stations was also obtained from the past five years to calculate the growth rate for the area during the buildout year (2023), which resulted in a $2.73 \%$ annual growth. The traffic data is provided in Attachment B.

## Trip Distribution

The peak hour distribution was determined for the trips generated by the proposed development. The trip distribution is consistent with existing traffic patterns from FDOT Traffic Online. The project trip distribution is shown below in Figure 2. The future buildout turning movement volumes for the site driveways were assigned based on the trip generation and distribution. The future buildout volumes are shown below in Figure 3.


Figure 2: Project Trip Distribution


Figure 3: Future Traffic Volumes

## Analysis

Guidelines provided by the NCHRP Report 457 Evaluating Intersection Improvements - An Engineering Study Guide were followed to evaluate the need for a northbound left-turn lane and a southbound right-turn lane on N Glades Trail. Right and left-turn lanes are not warranted on N Glades Trail. The turn lane warrant spreadsheets are included in Attachment C.

Guidelines provided by the FDOT Access Management Guidebook (2019) were followed to evaluate the need for an eastbound right-turn lane on US 98. The recommended guidelines state that for roadways with a posted speed limit of more than 45 miles per hour (mph), a threshold of 35 to 55 rightturns per hour should be met for exclusive right-turn lanes. The higher threshold of 55 right-turn movements per hour is for multilane highways. The projected eastbound right-turn volume on US 98 is expected to be 43 vehicles per hour during the PM peak hour under future buildout conditions. Therefore, the project is not anticipated to warrant an exclusive eastbound right-turn lane along US 98 based on FDOT guidelines.


## Kimley»)Horn

## Attachment A: Site Plan



## Kimley»)Horn

## Attachment B: Traffic Data

## Historic Traffic Data (Source: FDOT)

| Station | 460277 | AAGR |
| :---: | :---: | :---: |
| Route | US 98 W |  |
| Location | A |  |
| 2016 | 49,000 | $10.20 \%$ |
| 2017 | 54,000 | $-5.56 \%$ |
| 2018 | 51,000 | $2.94 \%$ |
| 2019 | 52,500 | $-23.81 \%$ |
| 2020 | 40,000 | $30.00 \%$ |
| 2021 | 52,000 | $2.76 \%$ |



| Station | 460207 |  |
| :---: | :---: | :---: |
| Route | R Jackson Blvd |  |
| Location | B |  |
| 2016 | 12,300 |  |
| 2017 | 13,600 | $10.57 \%$ |
| 2018 | 13,100 | $-3.68 \%$ |
| 2019 | 14,000 | $6.87 \%$ |
| 2020 | 12,300 | $-12.14 \%$ |
| 2021 | 16,800 | $36.59 \%$ |
|  |  |  |
|  |  | $7.64 \%$ |


| Station | 460203 |  |
| :---: | :---: | :---: |
| Route | US 98E |  |
| Location | C |  |
| 2016 | 40,500 |  |
| 2017 | 39,500 | $-2.47 \%$ |
| 2018 | 39,500 | $0.00 \%$ |
| 2019 | 44,500 | $12.66 \%$ |
| 2020 | 39,000 | $-12.36 \%$ |
| 2021 | 46,500 | $19.23 \%$ |




| Station | 460280 | AAGR |
| :---: | :---: | :---: |
| Route | SR 392A |  |
| Location | D |  |
| 2016 | 23,500 | $-4.26 \%$ |
| 2017 | 22,500 | $13.33 \%$ |
| 2018 | 25,500 | $7.84 \%$ |
| 2019 | 27,500 | $0.00 \%$ |
| 2020 | 27,500 | $-10.91 \%$ |
| 2021 | 24,500 | $1.20 \%$ |




HSA Consulting Group Inc.
1284 Jackson Avenue
Chipley FL, 32428
File Name : TMC SO~2
Site Code : 00000001
Start Date : 11/18/2020
Page No : 2
Groups Printed- Cars - Trucks

|  | SR 30 (US 98) Eastbound |  |  |  |  | SR 30 (US 98) Westbound |  |  |  |  | N Glades Trail Northbound |  |  |  |  | N Glades Trail Southbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Thru | Right | UTRN | App. Total | Left | Thru | Right | UTRN | App. Total | Left | Thru | Right | UTRN | App. Total | Left | Thru | Right | UTRN | App. Total | $\begin{aligned} & \text { Int. } \\ & \text { Total } \end{aligned}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  | 1.0 | 1.0 | 1.0 | 1.0 |  |  |
| 03:00 PM | 1 | 425 | 5 | 1 | 432 | 3 | 360 | 10 | 6 | 379 | 1 | 0 | 7 | 0 | 8 | 3 | 0 | 2 | 0 | 5 | 824 |
| 03:15 PM | 0 | 463 | 8 | 0 | 471 | 3 | 403 | 21 | 2 | 429 | 1 | 0 | 7 | 0 | 8 | 7 | 0 | 10 | 0 | 17 | 925 |
| 03:30 PM | 1 | 403 | 1 | 3 | 408 | 4 | 410 | 11 | 4 | 429 | 0 | 0 | 6 | 0 | 6 | 4 | 0 | 7 | 0 | 11 | 854 |
| 03:45 PM | 2 | 430 | 6 | 1 | 439 | 4 | 452 | 13 | 5 | 474 | 2 | 0 | 2 | 0 | 4 | 9 | 0 | 5 | 0 | 14 | 931 |
| Total | 4 | 1721 | 20 | 5 | 1750 | 14 | 1625 | 55 | 17 | 1711 | 4 | 0 | 22 | 0 | 26 | 23 | 0 | 24 | 0 | 47 | 3534 |
| 04:00 PM | 2 | 499 | 5 | 0 | 506 | 2 | 478 | 16 | 7 | 503 | 1 | 0 | 6 | 0 | 7 | 7 | 1 | 4 | 0 | 12 | 1028 |
| 04:15 PM | 2 | 478 | 5 | 0 | 485 | 7 | 416 | 19 | 1 | 443 | 1 | 0 | 4 | 0 | 5 | 9 | 0 | 8 | 0 | 17 | 950 |
| 04:30 PM | 0 | 453 | 19 | 0 | 472 | 4 | 402 | 23 | 2 | 431 | 4 | 0 | 10 | 0 | 14 | 6 | 0 | 5 | 0 | 11 | 928 |
| 04:45 PM | 1 | 453 | 3 | 0 | 457 | 5 | 401 | 10 | 5 | 421 | 3 | 0 | 5 | 0 | 8 | 6 | 1 | 5 | 0 | 12 | 898 |
| Total | 5 | 1883 | 32 | 0 | 1920 | 18 | 1697 | 68 | 15 | 1798 | 9 | 0 | 25 | 0 | 34 | 28 | 2 | 22 | 0 | 52 | 3804 |
| 05:00 PM | 2 | 489 | 10 | 1 | 502 | 6 | 400 | 18 | 2 | 426 | 2 | 0 | 9 | 0 | 11 | 6 | 1 | 5 | 0 | 12 | 951 |
| 05:15 PM | 2 | 476 | 3 | 0 | 481 | 12 | 417 | 18 | 5 | 452 | 1 | 0 | 2 | 0 | 3 | 6 | 0 | 5 | 0 | 11 | 947 |
| 05:30 PM | 3 | 461 | 7 | 0 | 471 | 9 | 385 | 13 | 4 | 411 | 2 | 0 | 2 | 0 | 4 | 5 | 0 | 3 | 0 | 8 | 894 |
| 05:45 PM | 1 | 431 | 2 | 1 | 435 | 8 | 304 | 14 | 4 | 330 | 2 | 0 | 0 | 0 | 2 | 4 | 0 | 2 | 0 | 6 | 773 |
| Total | 8 | 1857 | 22 | 2 | 1889 | 35 | 1506 | 63 | 15 | 1619 | 7 | 0 | 13 | 0 | 20 | 21 | 1 | 15 | 0 | 37 | 3565 |
| Grand Total | 57 | 15639 | 154 | 66 | 15916 | 173 | 15189 | 405 | 163 | 15930 | 76 | 4 | 210 | 0 | 290 | 323 | 13 | 219 | 0 | 555 | 32691 |
| Apprch \% | 0.4 | 98.3 | 1.0 | 0.4 |  | 1.1 | 95.3 | 2.5 | 1.0 |  | 26.2 | 1.4 | 72.4 | 0.0 |  | 58.2 | 2.3 | 39.5 | 0.0 |  |  |
| Total \% | 0.2 | 47.8 | 0.5 | 0.2 | 48.7 | 0.5 | 46.5 | 1.2 | 0.5 | 48.7 | 0.2 | 0.0 | 0.6 | 0.0 | 0.9 | 1.0 | 0.0 | 0.7 | 0.0 | 1.7 |  |

## Intersection Volume Worksheet

Hombre - Panama City Beach, Florida
US 98 and Driveway
PM PEAK HOUR

| Description | Driveway Northbound |  |  | Southbound |  |  | $\text { US } 98$ <br> Eastbound |  |  | US 98 <br> Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Through | Right | Left | Through | Right | Left | Through | Right | Left | Through | Right |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Existing 2021 PM Volumes |  |  |  |  |  |  |  | 1693 |  |  | 1839 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Annual Growth Rate |  |  | 2.7\% |  |  |  |  | 2.7\% | 2.7\% |  | 2.7\% |  |
| Growth Factor |  |  | 1.06 |  |  |  |  | 1.06 | 1.06 |  | 1.06 |  |
| Growth Trips |  |  |  |  |  |  |  | 95 |  |  | 103 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Background 2023 PM Volumes |  |  |  |  |  |  |  | 1788 |  |  | 1942 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Net New Trip Distribution IN |  |  |  |  |  |  |  |  | 50\% |  |  |  |
| Net New Trip Distribution OUT |  |  | 50\% |  |  |  |  |  |  |  |  |  |
| Net New Project Trips |  |  | 25 |  |  |  |  |  | 43 |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total Project Trips |  |  | 25 |  |  |  |  |  | 43 |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Future 2023 PM Volumes |  |  | 25 |  |  |  |  | 1788 | 43 |  | 1942 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

## Intersection Volume Worksheet

Hombre - Panama City Beach, Florida
Hombre - Panama City Beach, Florida

| N Glades and Driveway |
| :---: | :---: |
| PM PEAK HOUR |


|  | N Glades Trail Northbound |  |  | N Glades Trail Southbound |  |  | Driveway Eastbound |  |  | Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Description | Left | Through | Right | Left | Through | Right | Left | Through | Right | Left | Through | Right |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Existing 2020 PM Volumes |  | 34 |  |  | 52 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Annual Growth Rate | 2.7\% | 2.7\% |  |  | 2.7\% | 2.7\% | 2.7\% |  | 2.7\% |  |  |  |
| Growth Factor | 1.08 | 1.08 |  |  | 1.08 | 1.08 | 1.08 |  | 1.08 |  |  |  |
| Growth Trips |  | 3 |  |  | 4 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Background 2023 PM Volumes |  | 37 |  |  | 56 |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Net New Trip Distribution IN | 5\% |  |  |  |  | 45\% |  |  |  |  |  |  |
| Net New Trip Distribution OUT |  |  |  |  |  |  | 45\% |  | 5\% |  |  |  |
| Net New Project Trips | 4 |  |  |  |  | 38 | 23 |  | 2 |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total Project Trips | 4 |  |  |  |  | 38 | 23 |  | 2 |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Future 2023 PM Volumes | 4 | 37 |  |  | 56 | 38 | 23 |  | 2 |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |

## Kimley»"Horn

## Attachment C: Turn Warrant Spreadsheets

Figure 2-5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.
2-lane roadway (English)
INPUT

| Variable | Value |
| :--- | :---: |
| $85^{\text {th }}$ percentile speed, $\mathrm{mph}:$ | 25 |
| Percent of left-turns in advancing volume $\left(\mathrm{V}_{\mathrm{A}}\right), \%:$ | $10 \%$ |
| Advancing volume $\left(\mathrm{V}_{\mathrm{A}}\right)$, veh/h: | 41 |
| Opposing volume $\left(\mathrm{V}_{\mathrm{O}}\right)$, veh $/ \mathrm{h:}$ | 94 |

## OUTPUT

| Variable | Value |
| :--- | :---: |
| Limiting advancing volume $\left(\mathrm{V}_{\mathrm{A}}\right)$, veh/h: | 607 |
| Guidance for determining the need for a major-road left-turn bay: |  |
| Left-turn treatment NOT warranted. |  |



CALIBRATION CONSTANTS

| Variable | Value |
| :--- | :---: |
| Average time for making left-turn, s: | 3.0 |
| Critical headway, s: | 5.0 |
| Average time for left-turn vehicle to clear the advancing lane, s: | 1.9 |

Figure 2-6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.
INPUT

| Roadway geometry: | 2-lane roadw ay |  |
| :--- | :---: | :---: |
| Variable |  | Value |
| Major-road speed, mph: | 25 |  |
| Major-road volume (one direction), veh/h: | 94 |  |
| Right-turn volume, veh/h: | 38 |  |

OUTPUT

| Variable | Value |
| :--- | :---: |
| Limiting right-turn volume, veh/h: | 52779082 |
| Guidance for determining the need for a major-road <br> right-turn bay for a 2-lane roadway: |  |
| Do NOT add right-turn bay. |  |



