

### **MEMORANDUM**

To: The City of Panama City Beach

From: Kimley-Horn and Associates

Date: November 15, 2022

Subject: Hombre Apartments Traffic Analysis

### Purpose and Methodology

The purpose of this memorandum is to summarize the turn lane analysis prepared for the proposed multifamily housing development located southwest of the intersection of US 98 and N Glades Trail in Panama City Beach, Florida. The proposed development has two access points; the US 98 access is a right-in/right-out driveway, and the N Glades Trail access is a full access driveway. There is an additional access driveway on Coyote Pass which is not included in the analysis because of its restriction for emergency vehicles only. This memorandum provides the methodology and analysis for evaluating the need for an eastbound right-turn lane on US 98, as well as a northbound left-turn lane and a southbound right-turn lane on N Glades Trail. A map of the project location is shown below in **Figure 1**, and a site plan for the proposed development is available in **Attachment A**.



Figure 1: Project Location



### **Trip Generation**

The subject development is proposed to consist of 267 multifamily units. The Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11<sup>th</sup> Edition* was utilized to calculate the trip generation potential of the proposed development. ITE trip generation rates for Land Use Code 220 (Multifamily Housing [Low-Rise]) were applied to calculate the daily and PM peak hour trips anticipated from the proposed development. The development is expected to generate approximately 1,787 daily net new trips and 135 PM peak hour net new trips (85 entering, 50 exiting). **Table 1** summarizes the trip generation calculations.

**Table 1: Trip Generation Calculations** 

ITE	Land Use		onsity	Daily	PM Peak Hour			
LUC	Land Ose	U	ensity	Trips	Total	In	Out	
220	Multifamily Housing (Low-Rise)	267	Units	1,787	135	85	50	
	GROSS TRIPS			1,787	135	85	50	

#### Traffic Development

Existing traffic volumes were obtained from the Florida Department of Transportation (FDOT) *Florida Traffic Online (2021)* and from a previously submitted FDOT intersection analysis report completed at the site. The peak directional volumes for US 98 were retrieved from count station 460203, just east of the site. The seasonal factor corresponding to the week of data collection for US 98 was 0.99. Since the seasonal factor was less than 1.0, to provide for a conservative analysis, it was not applied. Historical traffic data from surrounding count stations was also obtained from the past five years to calculate the growth rate for the area during the buildout year (2023), which resulted in a 2.73% annual growth. The traffic data is provided in **Attachment B.** 

### **Trip Distribution**

The peak hour distribution was determined for the trips generated by the proposed development. The trip distribution is consistent with existing traffic patterns from FDOT *Traffic Online*. The project trip distribution is shown below in **Figure 2**. The future buildout turning movement volumes for the site driveways were assigned based on the trip generation and distribution. The future buildout volumes are shown below in **Figure 3**.



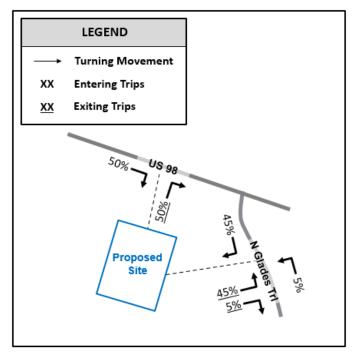
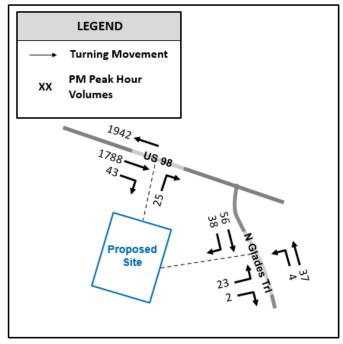


Figure 2: Project Trip Distribution



**Figure 3: Future Traffic Volumes** 



#### **Analysis**

Guidelines provided by the *NCHRP Report 457 Evaluating Intersection Improvements – An Engineering Study Guide* were followed to evaluate the need for a northbound left-turn lane and a southbound right-turn lane on N Glades Trail. Right and left-turn lanes are not warranted on N Glades Trail. The turn lane warrant spreadsheets are included in **Attachment C**.

Guidelines provided by the *FDOT Access Management Guidebook (2019)* were followed to evaluate the need for an eastbound right-turn lane on US 98. The recommended guidelines state that for roadways with a posted speed limit of more than 45 miles per hour (mph), a threshold of 35 to 55 right-turns per hour should be met for exclusive right-turn lanes. The higher threshold of 55 right-turn movements per hour is for multilane highways. The projected eastbound right-turn volume on US 98 is expected to be 43 vehicles per hour during the PM peak hour under future buildout conditions. Therefore, the project is not anticipated to warrant an exclusive eastbound right-turn lane along US 98 based on FDOT guidelines.

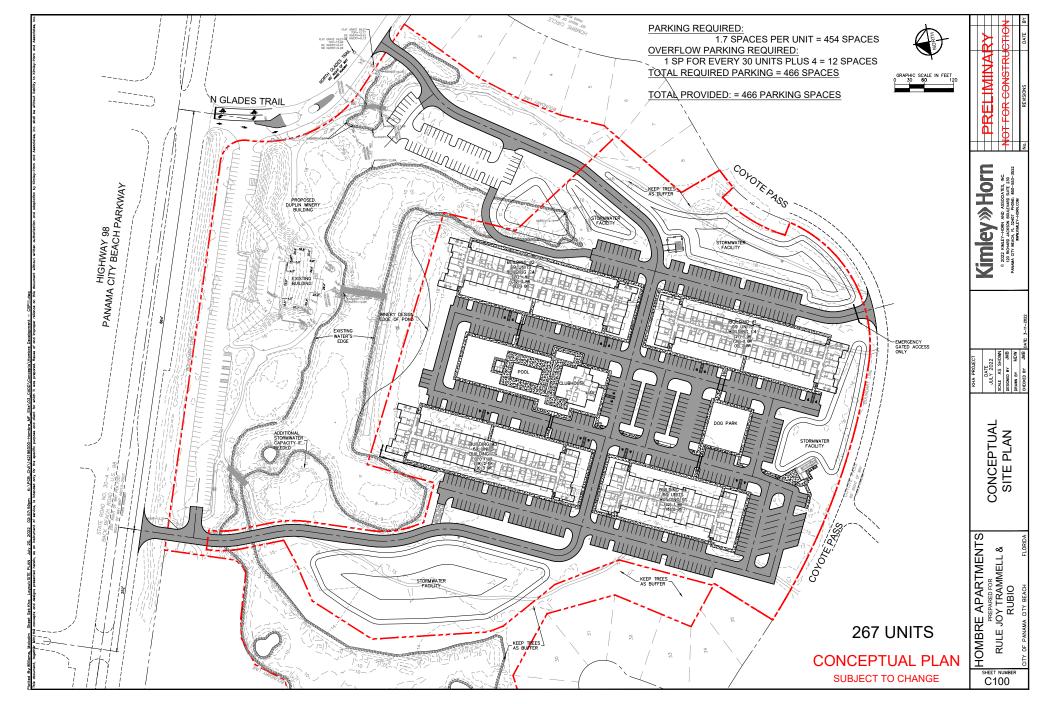


Ali H. Brighton, P.E. Florida Registration Number 77731 Kimley-Horn and Associates, Inc. 2619 Centennial Boulevard, Suite 200 Tallahassee, Florida 32308 Registry 35106 This document has been digitally signed and sealed by Ali Hanes Brighton, P.E. on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



**Attachment A: Site Plan** 





**Attachment B: Traffic Data** 

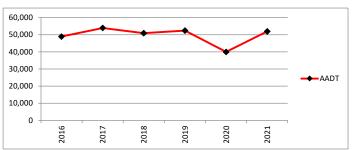
## **Historic Traffic Data (Source: FDOT)**

Station	460277	
Route	US 98 W	AAGR
Location	Α	
2016	49,000	
2017	54,000	10.20%
2018	51,000	-5.56%
2019	52,500	2.94%
2020	40,000	-23.81%
2021	52,000	30.00%
		2.76%

Station	460207					
Route	Route R Jackson Blvd					
Location	В					
2016	12,300					
2017	13,600	10.57%				
2018	13,100	-3.68%				
2019	14,000	6.87%				
2020	12,300	-12.14%				
2021	2021 16,800					
		7.64%				

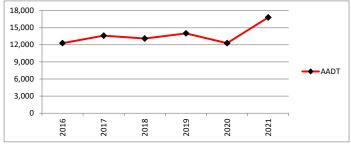
Station	460203					
Route	Route US 98 E					
Location	С					
2016	40,500					
2017	39,500	-2.47%				
2018	39,500	0.00%				
2019	44,500	12.66%				
2020	39,000	-12.36%				
2021	46,500	19.23%				
		3.41%				

Station	460280	
Route	SR 392A	AAGR
Location	D	
2016	23,500	
2017	22,500	-4.26%
2018	25,500	13.33%
2019	27,500	7.84%
2020	27,500	0.00%
2021	24,500	-10.91%
		1.20%

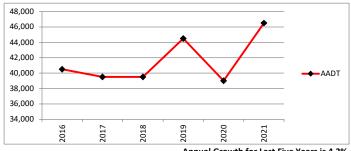


Annual Growth for Last Five Years is -0.9%

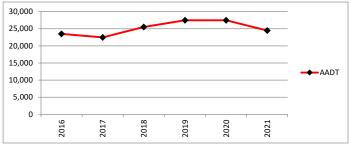
-0.90%



Annual Growth for Last Five Years is 5.4% 5.40%



Annual Growth for Last Five Years is 4.2% 4.20%



Annual Growth for Last Five Years is 2.2% 2.20%

AAGR 2.73%

COUNTY: 46 STATION: 0203

DESCRIPTION: US 98 (BACK BCH) - 425' E OF CAULEY AVE (W OF RAMP

START DATE: 09/02/2021

START TIME: 1400

DIRECTION: E DIRECTION: W COMBINED 1ST 2ND 3RD 4TH TOTAL 1ST 2ND 3RD 4TH TOTAL TOTAL TIME 
 44
 32
 45
 28
 149
 95
 68
 51
 41
 255

 27
 17
 22
 21
 87
 53
 45
 28
 22
 148

 17
 19
 21
 16
 73
 28
 25
 26
 21
 100

 28
 24
 33
 36
 121
 16
 16
 14
 29
 75

 40
 30
 72
 70
 212
 27
 27
 22
 39
 115

 68
 92
 135
 150
 445
 50
 72
 77
 105
 304

 105
 306
 126
 1456
 100
 164
 300
 300
 304
 255 | 404 363 1156 506 1865 402 1534 383 1750 315 1342 379 1469 366 1316 349 1561 378 1386 411 1543 437 1510 349 1451 379 1530 426 1599 381 1595 435 1717 493 1679 427 1693 453 1839 356 1640 424 1766 237 1190 319 1369 227 258 213 902 168 143 689 245 1205 191 187 202 821 149 143 641 1145 152 130 121 101 504 94 73 98 82 347 140 124 580 927 109 97 106 417 656 68 64 59 48 239

24-HOUR TOTALS: 24030 23591 47621

				INFORMATION		
	DIREC	TION: E	DIREC	TION: W	COMBINED	DIRECTIONS
	HOUR	VOLUME	HOUR	VOLUME	HOUR	VOLUME
A.M.	730	1932	715	1581	730	3506
P.M.	1515	1764	1545	1879	1545	3580
DAILY	730	1932	1545	1879	1545	3580

# HSA Consulting Group Inc. 1284 Jackson Avenue Chipley FL, 32428

File Name: TMC\_SO~2 Site Code: 00000001

Start Date : 11/18/2020

Page No : 2

Groups Printed- Cars - Trucks

										Filliteu- C	ais - 110										
		SI	R 30 (US	S 98)			SF	R 30 (US	8 98)			N	Glades 7	Trail			N	Glades 7	Trail		
			Eastbou	nd			1	Nestbou	ınd			1	Northbou	nd			S	Southbou	ınd		
Start Time	Left	Thru	Right	UTRN	App. Total	Left	Thru	Right	UTRN	App. Total	Left	Thru	Right	UTRN	App. Total	Left	Thru	Right	UTRN	App. Total	Int. Total
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
03:00 PM	1	425	5	1	432	3	360	10	6	379	1	0	7	0	8	3	0	2	0	5	824
03:15 PM	0	463	8	0	471	3	403	21	2	429	1	0	7	0	8	7	0	10	0	17	925
03:30 PM	1	403	1	3	408	4	410	11	4	429	0	0	6	0	6	4	0	7	0	11	854
03:45 PM	2	430	6	1	439	4	452	13	5	474	2	0	2	0	4	9	0	5	0	14	931
Total	4	1721	20	5	1750	14	1625	55	17	1711	4	0	22	0	26	23	0	24	0	47	3534
					•																
04:00 PM	2	499	5	0	506	2	478	16	7	503	1	0	6	0	7	7	1	4	0	12	1028
04:15 PM	2	478	5	0	485	7	416	19	1	443	1	0	4	0	5	9	0	8	0	17	950
04:30 PM	0	453	19	0	472	4	402	23	2	431	4	0	10	0	14	6	0	5	0	11	928
04:45 PM	1	453	3	0	457	5	401	10	5	421	3	0	5	0	8	6	1	5	0	12	898
Total	5	1883	32	0	1920	18	1697	68	15	1798	9	0	25	0	34	28	2	22	0	52	3804
					,					'					·						
05:00 PM	2	489	10	1	502	6	400	18	2	426	2	0	9	0	11	6	1	5	0	12	951
05:15 PM	2	476	3	0	481	12	417	18	5	452	1	0	2	0	3	6	0	5	0	11	947
05:30 PM	3	461	7	0	471	9	385	13	4	411	2	0	2	0	4	5	0	3	0	8	894
05:45 PM	1	431	2	1	435	8	304	14	4	330	2	0	0	0	2	4	0	2	0	6	773
Total	8	1857	22	2	1889	35	1506	63	15	1619	7	0	13	0	20	21	1	15	0	37	3565
					,					,					'						
Grand Total	57	15639	154	66	15916	173	15189	405	163	15930	76	4	210	0	290	323	13	219	0	555	32691
Apprch %	0.4	98.3	1.0	0.4		1.1	95.3	2.5	1.0		26.2	1.4	72.4	0.0		58.2	2.3	39.5	0.0		
Total %	0.2	47.8	0.5	0.2	48.7	0.5	46.5	1.2	0.5	48.7	0.2	0.0	0.6	0.0	0.9	1.0	0.0	0.7	0.0	1.7	
										,					1						

# **Intersection Volume Worksheet**

Hombre - Panama City Beach, Florida

# US 98 and Driveway PM PEAK HOUR

		Driveway  Northbound  Southbound				US 98 Eastbound		US 98 <u>Westbound</u>				
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Existing 2021 PM Volumes								1693			1839	
Annual Growth Rate			2.7%					2.7%	2.7%		2.7%	
Growth Factor			1.06					1.06	1.06		1.06	
Growth Trips								95			103	
Background 2023 PM Volumes								1788			1942	
Net New Trip Distribution IN									50%			
Net New Trip Distribution OUT			50%									
Net New Project Trips			25						43			
Total Project Trips			25						43			
										<u> </u>		
									·			
Future 2023 PM Volumes			25					1788	43		1942	

# **Intersection Volume Worksheet**

Hombre - Panama City Beach, Florida

# N Glades and Driveway PM PEAK HOUR

	N Glades Trail <u>Northbound</u>				N Glades Tra Southbound			Driveway <b>Eastbound</b>		Westbound		
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
Existing 2020 PM Volumes		34			52							
Annual Growth Rate	2.7%	2.7%			2.7%	2.7%	2.7%		2.7%			
Growth Factor	1.08	1.08			1.08	1.08	1.08		1.08			
Growth Trips		3			4							
Background 2023 PM Volumes		37			56							
Net New Trip Distribution IN	5%					45%						
Net New Trip Distribution OUT							45%		5%			
Net New Project Trips	4					38	23		2			
Total Project Trips	4					38	23		2			
Future 2023 PM Volumes	4	37			56	38	23		2			



**Attachment C: Turn Warrant Spreadsheets** 

Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

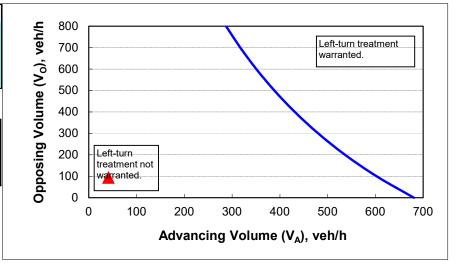
## 2-lane roadway (English)

## INPUT

Variable	Value
85 <sup>th</sup> percentile speed, mph:	25
Percent of left-turns in advancing volume (V <sub>A</sub> ), %:	10%
Advancing volume (V <sub>A</sub> ), veh/h:	41
Opposing volume (V <sub>0</sub> ), veh/h:	94

## OUTPUT

Variable	Value				
Limiting advancing volume (V <sub>A</sub> ), veh/h:	607				
Guidance for determining the need for a major-road left-turn bay:					
Left-turn treatment NOT warranted.					



### **CALIBRATION CONSTANTS**

Variable	Value
Average time for making left-turn, s:	3.0
Critical headway, s:	5.0
Average time for left-turn vehicle to clear the advancing lane, s:	1.9

Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

## INPUT

Roadway geometry:	2-lane roadw ay	
Variable		Value
Major-road speed, mph:		25
Major-road volume (one direction), veh/h:		94
Right-turn volume, veh/h:		38

## OUTPUT

Value		
52779082		
Guidance for determining the need for a major-road		
right-turn bay for a 2-lane roadway:		
Do NOT add right-turn bay.		

